

Technical Assignment 2
Cost and Methods Analysis



The Wilmer Eye Institute Outpatient Surgery & Laboratory Building
Baltimore, Maryland

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Construction Management

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Executive Summary

The key findings in this technical report are as follows:

- All interior tasks in schedule (walls, rough-ins, finishes, etc.) are grouped together by floor and type of space within the floor, reflecting the manner in which things will be constructed. The format/sequence of the schedule could potentially be an area that could be improved upon further investigation.
- Utilities have been run straight across North Broadway Street from the adjacent Weinberg Building.
- There is no area designated for material lay-down. All material deliveries will either be directly constructed into the building or staged within the building footprint.
- Exterior wall assembly consists primarily of face brick cavity wall with CMU block backup and double-pane ¼" insulating glazing with tubular aluminum mullions.
- Cast-in-place concrete building will require an estimated 12,650 cubic yards of concrete and 760 tons of rebar to construct the superstructure.

Overall, this report has provided a general technical familiarity with the building and its systems. Learning the many details of the different engineered systems has provided much insight into the construction processes that will occur, and will be extremely helpful in future analysis to come.



Temporary scaffold bridge over tunnel excavation

Schedule Narrative

The schedule compiled for this second technical report is notably more extensive than the previously submitted summary schedule. Activities related directly to the construction of the project (i.e. excluding design and procurement of services) have been expanded upon showing sub-tasks and their durations.

It is easily noticed with this format of schedule that the activities are organized by space, rather than by trade. This represents the fashion in which the building is to be constructed.

In general, the interior work will commence by floor from bottom to top as the superstructure is constructed, and work within the floor is organized by type of space (i.e. general sterile, laboratory, operating room, etc.).

The detailed schedule is located in the Appendix at the end of this document.



Looking North from Southeast corner of site.

Site Logistics

As with the detailed schedule, the site logistics plan included in this report expands upon the previously submitted plan while also highlighting the excavation process. As the plan illustrates, the major earthwork will commence to three different elevations, and all other non-shaded areas on the plan will be sloped accordingly.

Additionally included are the locations of the temporary subcontractor office trailers, temporary utility connections, tower crane, man/material hoist, and the construction ramp needed for excavation prior to the crane erection.

There will also be a temporary scaffold bridge for foot traffic over the tunnel trench throughout the duration of the tunnel construction. After the tunnel is completed the earth will be backfilled and the construction gate on Jefferson Street can function as a vehicle exit.

Temporary electric, as well as future permanent electric and water services have been routed straight across North Broadway Street from the adjacent Weinberg Building. This deviates from the original infrastructure utility documents. There is a fee associated with running utilities and like objects under public roadways, and this option essentially costs less because there is less length underneath the street. In the northeast corner of the site is the hub for the campus utilities located underground that will be the future connection point for all permanent HVAC lines.

Also note that there is no area designated for material lay-down. All material deliveries will either be directly constructed into the building or staged within the building footprint.

The updated site logistics plan can be found in the Appendix at the end of this document.



Temporary excavation ramp to be removed.

Exterior Enclosures Costs

Exterior Enclosures Summary			
Total Exterior Enclosures Costs:			
	Material	Installation	Total
Brick Face Cavity Wall	\$379,401	\$698,023	\$1,077,424
Tubular Aluminum Framing	\$284,325	\$266,938	\$551,262
Curtain Wall Glazing	\$187,163	\$194,323	\$381,486
Total	\$850,888	\$1,159,284	\$2,010,172

The exterior wall assembly consists of a brick face cavity wall and glass curtain wall glazing with tubular aluminum framing. The cavity wall primarily consists of standard bricks with an 8" CMU back-up, 2" polystyrene insulation and a 2" air space.

For simplicity in this estimate, it was assumed that all curtain wall glazing is double-pane ¼" insulated glass. Cavity wall figures include a brick shelf, ties to the backups and necessary dampproofing, flashing, and control joints every 20'. All unit prices were referenced from R.S. Means Assemblies Cost Data 2008.

Square Foot Extrapolation	
Exterior Enclosures Costs Per S.F. of Building Floor Area:	
Material	\$4.21
Installation	\$5.73
Total	\$9.93

A more detailed estimate broken down by elevation can be found in the Appendix at the end of this document.

Detailed Structural Systems Costs

Structural Systems Summary	
Total Structure Costs:	
Material	\$2,240,986
Labor	\$598,658
Equipment	\$48,865
Total	\$288,509
Total w/ Overhead & Profit	\$3,508,646

The superstructure of the Wilmer Eye Institute building consists of a mildly reinforced two-way concrete slab system with drop panels around the columns. Typical slab depth is 9½" with 5½" drop panels at the interior columns and 7½" drop panels at the exterior columns. Typical column size is 21"x 21" from the third floor up, and 24"x 24" in the basement and first floor. The foundation walls are cast-in-place concrete as well as the mat foundations and strip wall footings.

The building will require an estimated 12,650 cubic yards of concrete and 760 tons of rebar.

Square Foot Extrapolation	
Structural System Costs Per S.F. of Building Floor Area:	
Material	\$11.07
Labor	\$2.96
Equipment	\$0.24
Total	\$14.27
Total (O&P)	\$17.34

The detailed structural estimate and calculations can be found in the Appendix at the end of this document.

General Conditions Costs

General Conditions Summary	
Personnel	\$1,425,173
Trash Removal	\$101,000
Temporary Building Enclosure	\$63,240
Temporary Facilities/Utilities	\$52,552
Other	\$168,244
Total	\$1,810,209

General conditions for this project are fairly straightforward. A few items of interest are the scaffolding bridge for foot traffic over the tunnel trench, and the temporary building enclosure so interior work can commence in the winter of 2009.

Personnel salaries were estimated based on average numbers from R.S. Means 2008, and their durations on the project were established by the area in which each staff member is managing (i.e. shell/core manager is on for the scheduled duration of the shell/core construction).

Monthly Cost Breakdown	
Category	Cost per Month
Personnel	\$56,080
Office Supplies	\$415
Temporary Offices	\$700
Temporary Utilities	\$527
Trash Disposal	\$2,760
Temporary Facilities	\$685
Total Monthly Costs	\$61,167

A detailed breakdown of the general conditions costs can be found in the Appendix at the end of this document.

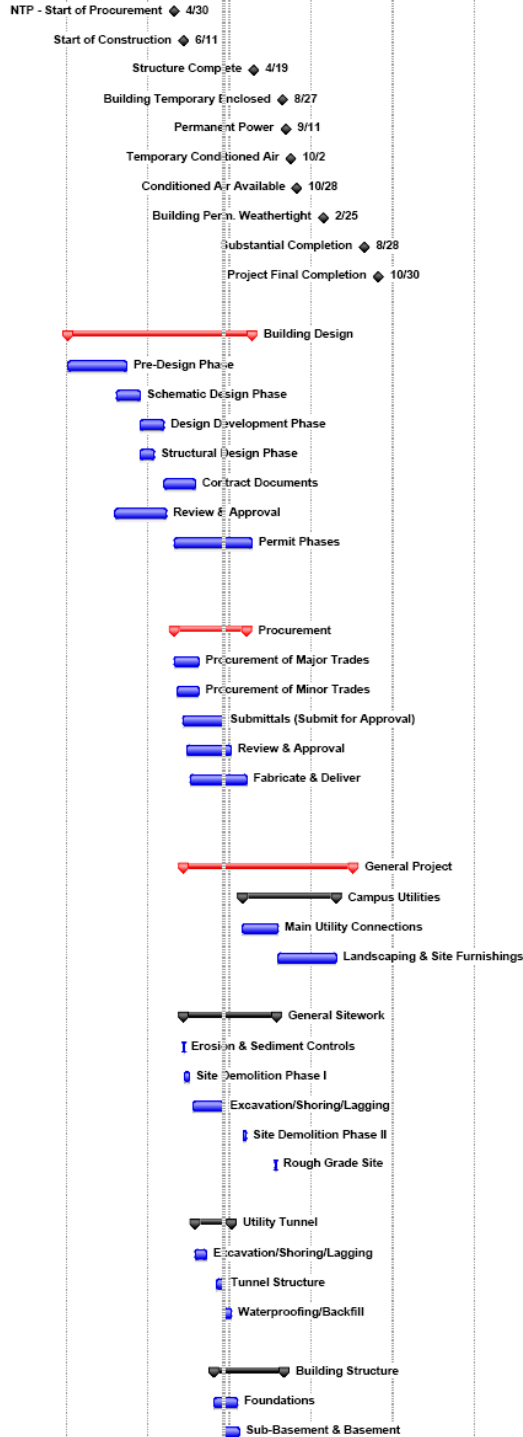
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Detailed Project Schedule

The Wilmer Eye Institute Outpatient Surgery & Lab Building

ID	Task Name	Duration	Start	Finish	2005		2006		2007		2008		2009		2010		2011		2012	
					Oct	Jan	Apr	Jul	Oct	Jan	Apr	Jul	Oct	Jan	Apr	Jul	Oct	Jan	Apr	Jul
1	Major Project Milestones	658 days	Mon 4/30/07	Fri 10/30/09																
2	NTP - Start of Procurement	0 days	Mon 4/30/07	Mon 4/30/07																
3	Start of Construction	0 days	Mon 6/11/07	Mon 6/11/07																
4	Structure Complete	0 days	Sat 4/19/08	Sat 4/19/08																
5	Building Temporary Enclosed	0 days	Wed 8/27/08	Wed 8/27/08																
6	Permanent Power	0 days	Thu 9/11/08	Thu 9/11/08																
7	Temporary Conditioned Air	0 days	Thu 10/2/08	Thu 10/2/08																
8	Conditioned Air Available	0 days	Tue 10/28/08	Tue 10/28/08																
9	Building Perm. Weathertight	0 days	Wed 2/25/09	Wed 2/25/09																
10	Substantial Completion	0 days	Fri 8/28/09	Fri 8/28/09																
11	Project Final Completion	0 days	Fri 10/30/09	Fri 10/30/09																
12																				
13	Building Design	592 days?	Mon 1/9/06	Fri 4/11/08																
14	Pre-Design Phase	190 days?	Mon 1/9/06	Fri 8/29/06																
15	Schematic Design Phase	77 days?	Mon 8/14/06	Tue 11/28/06																
16	Design Development Phase	78 days?	Tue 11/28/06	Thu 3/15/07																
17	Structural Design Phase	46 days?	Tue 11/28/06	Tue 1/30/07																
18	Contract Documents	101 days?	Thu 3/15/07	Thu 8/2/07																
19	Review & Approval	165 days?	Tue 8/8/06	Mon 3/28/07																
20	Permit Phases	252 days?	Mon 4/30/07	Fri 4/11/08																
21																				
22																				
23	Procurement	235 days?	Mon 4/30/07	Wed 3/19/08																
24	Procurement of Major Trades	80 days?	Mon 4/30/07	Fri 8/17/07																
25	Procurement of Minor Trades	70 days?	Mon 5/14/07	Fri 8/17/07																
26	Submittals (Submit for Approval)	129 days?	Fri 6/8/07	Wed 12/6/07																
27	Review & Approval	144 days?	Mon 6/25/07	Wed 1/6/08																
28	Fabricate & Deliver	183 days?	Wed 7/11/07	Wed 3/19/08																
29																				
30																				
31	General Project	547 days?	Mon 6/11/07	Thu 7/9/08																
32	Campus Utilities	302 days?	Sat 3/1/08	Fri 4/24/08																
33	Main Utility Connections	115 days?	Sat 3/1/08	Wed 8/6/08																
34	Landscaping & Site Furnishings	187 days?	Thu 8/7/08	Fri 4/24/09																
35																				
36	General Sitework	301 days?	Mon 6/11/07	Wed 7/30/08																
37	Erosion & Sediment Controls	4 days?	Mon 6/11/07	Thu 6/14/07																
38	Site Demolition Phase I	16 days?	Fri 6/15/07	Fri 7/6/07																
39	Excavation/Shoring/Lagging	98 days?	Mon 7/23/07	Wed 12/6/07																
40	Site Demolition Phase II	10 days?	Tue 3/4/08	Mon 3/17/08																
41	Rough Grade Site	5 days?	Thu 7/24/08	Wed 7/30/08																
42																				
43	Utility Tunnel	118 days?	Wed 8/1/07	Thu 1/10/08																
44	Excavation/Shoring/Lagging	37 days?	Wed 8/1/07	Thu 9/20/07																
45	Tunnel Structure	25 days?	Tue 11/6/07	Sat 12/8/07																
46	Waterproofing/Backfill	24 days?	Mon 12/10/07	Thu 1/10/08																
47																				
48	Building Structure	227 days?	Thu 10/25/07	Tue 9/2/08																
49	Foundations	76 days?	Thu 10/25/07	Wed 2/6/08																
50	Sub-Basement & Basement	54 days?	Tue 12/4/07	Thu 2/14/08																



Project: Wilmer Eye Institute Detailed

Task		Milestone		External Tasks	
Split		Summary		External Milestone	
Progress		Project Summary		Deadline	

Exterior Building Enclosure Estimate

The Wilmer Eye Institute Outpatient Surgery and Laboratory Building

Quick Building Stats: \$65M Cost of Construction
7 Storeys, 202,000 SF

Material Description	Quantity	Units	Material	Installation	Total	Material	Installation	Total
East Elevation								
Brick Face Cavity Wall Standard face brick, 8" conc. block backup Polystyrene cavity insulation	6620	S.F.	10.30	18.95	29.25	68,186	125,449	193,635
Tubular Aluminum Framing For 1/4" glass, one intermediate horizontal	9830	S.F.	13.90	13.05	26.95	136,637	128,282	264,919
Curtain Wall Panels Glazing panel, insulating, 1/2" thick, 2 lites	9830	S.F.	9.15	9.50	18.65	89,945	93,385	183,330
South Elevation								
Brick Face Cavity Wall Standard face brick, 8" conc. block backup Polystyrene cavity insulation	5790	S.F.	10.30	18.95	29.25	59,637	109,721	169,358
Tubular Aluminum Framing For 1/4" glass, one intermediate horizontal	4280	S.F.	13.90	13.05	26.95	59,492	55,854	115,346
Curtain Wall Panels Glazing panel, insulating, 1/2" thick, 2 lites	4280	S.F.	9.15	9.50	18.65	39,162	40,660	79,822
West Elevation								
Brick Face Cavity Wall Standard face brick, 8" conc. block backup Polystyrene cavity insulation	16855	S.F.	10.30	18.95	29.25	173,607	319,402	493,009
Tubular Aluminum Framing For 1/4" glass, one intermediate horizontal	3435	S.F.	13.90	13.05	26.95	47,747	44,827	92,573
Curtain Wall Panels Glazing panel, insulating, 1/2" thick, 2 lites	3435	S.F.	9.15	9.50	18.65	31,430	32,633	64,063
North Elevation								
Brick Face Cavity Wall Standard face brick, 8" conc. block backup Polystyrene cavity insulation	7570	S.F.	10.30	18.95	29.25	77,971	143,452	221,423
Tubular Aluminum Framing For 1/4" glass, one intermediate horizontal	2910	S.F.	13.90	13.05	26.95	40,449	37,976	78,425
Curtain Wall Panels Glazing panel, insulating, 1/2" thick, 2 lites	2910	S.F.	9.15	9.50	18.65	26,627	27,645	54,272
Totals:						Material	Installation	Total
						\$850,888	\$1,159,284	\$2,010,172

*Cavity wall assembly includes brick shelf, ties to the backups and necessary dampproofing, flashing, and control joints every 20'.

*Figures referenced from R.S. Means Assemblies Cost Data 2008

Detailed Structural Systems Estimate

The Wilmer Eye Institute Outpatient Surgery & Lab Building

Quick Building Stats: \$65M Cost of Construction
7 Storeys, 202,000 SF

Description	Quantity	Units	Material	Labor	Equipment	Total	Total (O&P)	Material	Labor	Equipment	Total	Total (O&P)	
Slab on Grade													
WWF 6 x 6 - W2.1 x 2.1 (8 x 8) 30 lb. per C.S.F.	286	CSF	15.40	21.50		36.90	53.50	4,397	6,138	0	10,535	15,274	
Normal Weight Concrete, 3,000 psi	440	CY	106.00			106.00	116.00	46,640	0	0	46,640	51,040	
Concrete placement, slab on grade, pumped	440	CY		15.30	5.80	21.10	31.50	0	6,732	2,552	9,284	13,860	
Slab on grade, incl. troweled finish, not incl. formwork or reinforcing, over 10,000 S.F., 6" thick	28550	SF	2.01	0.72	0.01	2.74	3.37	57,386	20,556	286	78,227	96,214	
											Subtotal:	144,686	176,388
Mat Footings													
Normal Weight Concrete, 3,000 psi	647	CY	104.00			104.00	114.00	67,246	0	0	67,246	73,712	
Concrete placement, foundation mats, over 2' thick	647	CY		4.97	1.88	6.85	10.15	0	3,214	1,216	4,429	6,563	
Reinforcing steel, footings, #4-#7	9820	Lb	0.47	0.31		0.78	1.05	4,615	3,044	0	7,660	10,311	
Reinforcing steel, footings, #8-#18	37260	Lb	0.47	0.18		0.65	0.82	17,512	6,707	0	24,219	30,553	
											Subtotal:	103,554	121,140
Pile Caps													
Normal Weight Concrete, 5,000 psi	102	CY	114.00			114.00	125.00	11,639	0	0	11,639	12,763	
Concrete placement, pile caps, over 10 C.Y., 12" thick	102	CY		8.30	3.13	11.43	16.95	0	847	320	1,167	1,731	
Reinforcing steel, footings, #8-#18	9960	Lb	0.47	0.18		0.65	0.82	4,681	1,793	0	6,474	8,167	
											Subtotal:	19,280	22,660
Column Footings													
Normal Weight Concrete, 8,000 psi	701	CY	212.00			212.00	233.00	148,612	0	0	148,612	163,333	
Concrete placement, spread footings, over 5' thick	701	CY		13.25	5.00	18.25	27.00	0	9,288	3,505	12,793	18,927	
Reinforcing steel, footings, #8-#18	41300	Lb	0.47	0.18		0.65	0.82	19,411	7,434	0	26,845	33,866	
											Subtotal:	188,250	216,126
Wall Footings													
Normal Weight Concrete, 3,000 psi	516	CY	104.00			104.00	114.00	53,664	0	0	53,664	58,824	
Concrete placement, footings, continuous, spread	516	CY		13.25	5.00	18.25	27.00	0	6,837	2,580	9,417	13,932	
Reinforcing steel, footings, #4-#7	23880	Lb	0.47	0.31		0.78	1.05	11,224	7,403	0	18,626	25,074	
											Subtotal:	81,707	97,830
Foundation Walls													
Normal Weight Concrete, 4,000 psi	1506	CY	108.00			108.00	119.00	162,648	0	0	162,648	179,214	
Concrete placement, walls, pumped	1506	CY		14.50	0.43	14.93	24.50	0	21,837	648	22,485	36,897	
Reinforcing steel, walls, #3-#7	78520	Lb	0.47	0.22		0.69	0.89	36,904	17,274	0	54,179	69,883	
Reinforcing steel, walls, #8-#18	158460	Lb	0.47	0.17		0.64	0.79	74,476	26,938	0	101,414	125,183	
											Subtotal:	340,726	411,177
Interior Basement Walls													
Normal Weight Concrete, 4,000 psi	102	CY	108.00			108.00	119.00	11,016	0	0	11,016	12,138	
Concrete placement, walls, pumped	102	CY		14.50	0.43	14.93	24.50	0	1,479	44	1,523	2,499	
Reinforcing steel, walls, #3-#7	7400	Lb	0.47	0.22		0.69	0.89	3,478	1,628	0	5,106	6,586	
CMU Wall, solid, reinforced alternate course	11570	SF	3.27	3.81		7.08	9.80	37,834	44,082	0	81,916	113,386	
											Subtotal:	99,560	134,609
Shear Walls													
Normal Weight Concrete, 5,000 psi	709	CY	114.00			114.00	125.00	80,826	0	0	80,826	88,625	
Concrete placement, walls, pumped	709	CY		14.50	0.43	14.93	24.50	0	10,281	305	10,585	17,371	
Reinforcing steel, walls, #3-#7	44000	Lb	0.47	0.22		0.69	0.89	20,680	9,680	0	30,360	39,160	
Reinforcing steel, walls, #8-#18	60520	Lb	0.47	0.17		0.64	0.79	28,444	10,288	0	38,733	47,811	
											Subtotal:	160,504	192,966
Columns													
Normal Weight Concrete, 5,000 psi	1093	CY	114.00			114.00	125.00	124,556	0	0	124,556	136,575	
Concrete placement, columns, pumped	1093	CY		21.50	8.15	29.65	44.50	0	23,491	8,905	32,396	48,621	
Reinforcing steel, columns, #3-#7	54422	Lb	0.47	0.44		0.91	1.26	25,578	23,946	0	49,524	68,572	
Reinforcing steel, columns, #8-#18	261446	Lb	0.47	0.29		0.76	1.00	122,880	75,819	0	198,699	261,446	
											Subtotal:	405,175	515,213
Elevated Slabs													
Normal Weight Concrete, 3,000 psi	6836	CY	104.00			104.00	114.00	710,944	0	0	710,944	779,304	
Concrete placement, slabs over 10" thick, pumped	6836	CY		11.05	4.17	15.22	22.50	0	75,538	28,506	104,044	153,810	
Reinforcing steel, elevated slabs, #4-#7	578540	Lb	0.49	0.23		0.72	0.93	283,485	133,064	0	416,549	538,042	
Reinforcing steel, elevated slabs, #8-#18	149380	Lb	0.47	0.29		0.76	1.00	70,209	43,320	0	113,529	149,380	
											Subtotal:	1,345,066	1,620,536
								Total Material	Total Labor	Total Equip.	Total	Total (O&P)	
								\$2,240,986	\$598,658	\$48,865	\$4,431,953	\$5,396,755	

*Assuming area well wall reinforcement is similar to typical foundation wall

*Assuming all foundation walls are 34'-0" due to conflicts in structural and architectural drawings

*Assuming typical dowel length of 4'-0"

*Assuming similar elevated slabs aside from deduction for atrium space

*Assuming all concrete is pumped

*All unit-cost data referenced from R.S. Means Facilities Construction Cost Data 2007, 22nd Annual Edition

General Conditions Cost Estimate

The Wilmer Eye Institute Outpatient Surgery & Laboratory Building

Quick Building Stats: \$65M Cost of Construction
7 Storeys, 202,000 SF

Category	Description	\$/Unit	Unit	Qty.	Total
Personnel	Project Executive	10000	mo	2	\$20,000
	Senior Project Manager	8333	mo	31	\$258,323
	Shell/Core PM	7500	mo	17	\$127,500
	Finishes PM	7500	mo	14	\$105,000
	MEP Manager	7500	mo	15	\$112,500
	Labs/OR Manager	7500	mo	14	\$105,000
	Senior Superintendent	7500	mo	15	\$112,500
	Superintendent	7100	mo	31	\$220,100
	Asst. Superintendent	6250	mo	31	\$193,750
	Project Engineer	5500	mo	17	\$93,500
Project Engineer	5500	mo	14	\$77,000	
Office Supplies	General	95	mo	31	\$2,945
	Office Elec./HVAC	110	mo	31	\$3,410
	Phone Bills	210	mo	31	\$6,510
Temporary Offices	Trailer Rental	700	mo	31	\$21,700
Temporary Utilities	Water	62	mo	31	\$1,922
	Electric	75	mo	31	\$2,325
	Lighting	15	CSF	202	\$3,030
	Heating	390	mo	6	\$2,340
Trash Disposal	Dumpsters	690	wk	140	\$96,600
	Trash Chutes	55	LF	80	\$4,400
Temporary Facilities	Toilet	685	mo	31	\$21,235
Temporary Fencing	Chain link, 11 ga, 6' high	7.15	LF	1120	\$8,008
Signage		16.55	SF	20	\$331
Scaffold Bridge	Catwalk, 10' span	190	ea	1	\$190
Surveying		1200	LS	1	\$1,200
Temporary Building Enclosure	Framing over openings	1	SF	63240	\$63,240
Photographic Documentation	Photographs	450	set	2	\$900
	Cameraman/Film	1375	visit	2	\$2,750
Scheduling	CPM Scheduling	15000	LS	1	\$15,000
Construction Clean-up		115000	LS	1	\$115,000
Man/Material Hoist		6000	LS	1	\$6,000
Miscellaneous/Unforeseen		6,000	LS	1	\$6,000
Total:					\$1,810,209

insert site plan here

Exterior Enclosures Estimate Calculations

Exponent
 Future Analysis Associates*
 Name _____ Date _____ Page 2 of _____
 Assignment _____ Class _____

SOUTH ELEVATION

TOTAL AREA:

$$[(32.1') + (32') \times 2 + (23.5')] \times [85'] = 10,070 \text{ SF}$$

WINDOWS:

CW-2 -- (7' x 11') (x 36) ^{-2 INT. HORIZ.} _{-1 VERT.}

CW-4 -- (11.7' x 76.7')

TOTAL CW AREA = 4280 SF

TOTAL CURT. WALL AREA = 5790 SF

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Exponent
 Future Analysis Associates*
 Name TYLER M. SMITH Date _____ Page 1 of _____
 Assignment EXTERIOR ENCLOSURE EST. CLASS

EAST ELEVATION

TOTAL AREA (EXCL. NE CW):

$$[(6.1') + (10.7') + (6.1') + (14.8') + (21') \times 8] \times [80'] = 16,450 \text{ SF}$$

WINDOWS:

CW-11 -- (7' x 11') (x 80) ^{-2 INT. HORIZ.} _{-1 INT. VERT.}

CW-12 -- (4' x 77.3')

CW-13 -- (3.8' x 77.3')

CW-16 (x 2) -- $\frac{(10' \times 77.3')}{2}$

CW-15 -- $\frac{(19.5' \times 77.3')}{2}$

CW-6 -- (7' x 54.5')

TOTAL CW AREA = 9830 SF

TOTAL CURT. WALL AREA = 6620 SF

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Exterior Enclosures Estimate Calculations

Exponent
Failure Analysis Associates*
Name _____ Date _____ Page 4 of _____
Assignment _____ Class _____

NORTH ELEVATION

TOTAL AREA (EXCL. NE CW):

$$[(105)(46.2') + (215')(92') + (42')(82')] = 10,480 \text{ SF}$$

WINDOWS:

CW-9 -- $(7.5' \times 14.8')$
 CW-10 -- $(7.5' \times 54.7')$
 CW-7 -- $(7.2' \times 10.7') (x2)$
 CW-8 -- $(19.7' \times 74.7')$
 *CW-14 -- $(10' \times 77.3')$

TOTAL CW AREA = 2910 SF
 TOTAL CAR. WALL AREA = 7570 SF

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Exponent
Failure Analysis Associates*
Name _____ Date _____ Page 3 of _____
Assignment _____ Class _____

WEST ELEVATION

TOTAL AREA:

$$[(12.5') + (20.5') + (21') \times 8 + (14.8') + (6.1') + (10.7') + (6.1')] \times [85'] = 20290 \text{ SF}$$

WINDOWS/LOUVERS:

CW-11 -- $(7' \times 11') (x 20)$
 CW-3 -- $(13.5' \times 8.5') (x 5)$
 CW-3 -- $(7' \times 11') (x 10)$
 CW-1 -- $(10.5' \times 10.5') (x 5)$

TOTAL CW AREA = 3435 SF
 TOTAL CAR. WALL AREA = 16,855 SF

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Structural Estimate Calculations

MAT FOOTINGS - 3,000 PSI (SHE WALLS - 4,000 PSI)

#1 $(22.5' \times 10.5') \times 48" \text{ THK} = 1845 \text{ CF} = \boxed{68 \text{ CY}}$
 #1 @ 10" O.C. EA WAY B
 #5 @ " " " " T

#3 $\#7 = (23' \times 20.5') + 2(12.5') \times (10.45' \text{ HIF}) = 985.125 = \boxed{0.5 \text{ TONS}}$
#8 $\#18 = (3.4' \text{ TCF}) + 32(10.5' \times 1.6' \text{ TONS})$

#3 $\#7 = (34.5' \times 21.5') \times 54" \text{ THK} = 4114 \text{ CF} = \boxed{153 \text{ CY}}$
 #10 @ 10" O.C. EA WAY B
 #5 @ 12" O.C. " " T

#8 $\#18 = (42(26.5') + 32(64.5')) \times (4.303' \text{ TH}) = 9540 \text{ LB} = \boxed{4.77 \text{ TONS}}$
#3 $\#7 = (35(26.5') + 27(31.5')) \times (10.43' \text{ TH}) = 1439 = \boxed{0.97 \text{ TONS}}$

#3 $\#7 = (21.5' \times 35.5') \times 54" \text{ THK} = 4394 \text{ CF} = \boxed{63 \text{ CY}}$
 #10 @ 10" O.C. EA WAY B
 #5 @ 12" O.C. " " T

#3 $\#7 = (36(27.5') + 28(35.5')) \times (10.43' \text{ TH}) = 2069 = \boxed{1.03 \text{ TONS}}$
#8 $\#18 = (3(27.5') + 33(35.5')) \times (4.303' \text{ TH}) = 8537 \text{ LB} = \boxed{4.23 \text{ TONS}}$

#3 $\#7 = (14' \times 31.4') \times 48" \text{ THK} = 2009.6 \text{ CF} = \boxed{75 \text{ CY}}$
 #10 @ 10" O.C. B } 31.4'
 #5 @ 12" O.C. T }
 #1 @ 12" O.C. B } 16'
 #5 @ 12" O.C. T }

#3 $\#7 = (16(31.4') + 32(16')) \times (10.43' \text{ TH}) = \boxed{0.53 \text{ TONS}}$
#8 $\#18 = (4308) = \boxed{2.2 \text{ TONS}}$

FOUNDATION/BASEMENT

SOC 5" THK w/ 6x6-110x110 WAF - 3500 PSI

AREA: 65 x 9 = 585
 66 x 9 = 594
 67 x 9 = 603
 68 x 9 = 612
 69 x 9 = 621
 70 x 9 = 630
 71 x 9 = 639
 72 x 9 = 648
 73 x 9 = 657
 74 x 9 = 666
 75 x 9 = 675
 76 x 9 = 684
 77 x 9 = 693
 78 x 9 = 702
 79 x 9 = 711
 80 x 9 = 720
 81 x 9 = 729
 82 x 9 = 738
 83 x 9 = 747
 84 x 9 = 756
 85 x 9 = 765
 86 x 9 = 774
 87 x 9 = 783
 88 x 9 = 792
 89 x 9 = 801
 90 x 9 = 810
 91 x 9 = 819
 92 x 9 = 828
 93 x 9 = 837
 94 x 9 = 846
 95 x 9 = 855
 96 x 9 = 864
 97 x 9 = 873
 98 x 9 = 882
 99 x 9 = 891
 100 x 9 = 900

28,700 SF - 150 SF = 28,550 SF
 $\frac{28,550 \text{ SF}}{140 \text{ CF}} = 204 \text{ CY}$

Structural Estimate Calculations

MAT FIGS (CONT.) - 3,000 PSI (THK WALL - 1100 PSI)

$(15' \times 27') \times 48'' \text{ THK} = 66 \text{ CY}$
 #10 @ 9" O.C. B } 15'
 #5 @ 12" O.C. T }
 #11 @ 12" O.C. B } 27'
 #5 @ 12" O.C. T }

#3-7 = $[8(15') + 15(27')] (1.143) = 0.47 \text{ TONS}$
 #8-18 = $[20(15') + 20(27')] (4.303) = 2.23 \text{ TONS}$

$(28.4') \times 45'' \text{ THK} = 97 \text{ CY}$
 #10 @ 9" O.C. B } 28.4'
 #5 @ 12" O.C. T }
 #13 @ 10" O.C. B } 23'
 #5 @ 12" O.C. T }

#3-7 = $[23(28.4') + 23(23')] (1.043) = 0.69 \text{ TONS}$
 #8-18 = $[30(23') + 28(28.4')] (4.303) = 3.6 \text{ TONS}$

PILE CAPS - 5000 PSI

$(26.4') \times 48'' \text{ THK} = 43.4 \text{ CY}$
 #10 @ 9" O.C. EA WAY B
 #8 @ 12" O.C. T

#8-18 = $[50(9.8') + 13(26.4')] (4.303) + [10(22.4') + 29(9.8')] (2.570) = 2.35 \text{ TONS}$

$(20.5') \times 58'' \text{ THK} = 58.7 \text{ CY}$
 #10 @ 9" O.C. EA WAY B
 #8 @ 12" O.C. T

#8-18 = $[35(15') + 14(20.5')] (4.303) + [12(28.5') + 29(11.5')] (2.67) = 2.63 \text{ TONS}$

Col. FIGS - 8,000 PSI

$8F-13 \times 11$
 $(13') \times (15') \times 48'' \text{ THK} \Rightarrow 241 \text{ CY}$
 12 #10 EA WAY B
 #8-18 = $24(13') (4.303) (11) = 7.4 \text{ TONS}$

$8F-14 \times 9$
 $(14') \times (14') \times 48'' \text{ THK} \Rightarrow 256 \text{ CY}$
 13 #10 EA WAY B
 #8-18 = $26(14') (4.303) (9) = 7.05 \text{ TONS}$

$8F-15 \times 6$
 $(15') \times (15') \times 48'' \text{ THK} \Rightarrow 204 \text{ CY}$
 11 #10 EA WAY B
 #8-18 = $32(15') (4.303) (6) = 6.20 \text{ TONS}$

Structural Estimate Calculations

WALL FIGS

> UNDER SLAB STEP DOWN ** ASSUMING FIG DOES NOT REST ON TOP OF COL. FIGS

LENGTH: $(12.5') \times (92') = 1151 \text{ CY}$

7 #4 CONTINUOUS W/ LENGTH

#5 @ 12" O.C. T

#4 @ 36" O.C. B

#3-7 = $7(92') \times (0.668) + 12(32') \times (1.043) + 3(3.2') \times (0.668)$

= 0.511 TONS

-VERT

$(6') \times (0.8') \times (92') = 46.4 \text{ CY}$

2 #4 @ 12" O.C. LENGTHWISE (ON FACE)

2 #4 @ 12" O.C. VERT -- (7' TO MATCH INTO FIG)

#3-7 = $12(42') \times (0.668) + 92(7') \times (0.668) = 0.58 \text{ TONS}$

> WF-2

LENGTH: $(7') + (9') + (7') + (8.6') + (2') + (18') = 274 \text{ CY}$

EXTRA LENGTH FOR REINF. THRU MATS: $(20.5') + (26.5') + (32') + (11.5') + (16') + (23')$

DIM: $(4') \times (3')$

#7 CONT. W/ LENGTH

#4 @ 48" O.C. STIRR.

#3-7 = $7(188.7) \times (2.044) + \left[\frac{(188.7)}{4} \times (4') \times (0.668) \right] = 1.38 \text{ TONS}$

> WF-1

LENGTH: $(16.7') = 31 \text{ CY}$

DIM: $(1') \times (2.5')$

#7 CONTINUOUS

#3-7 = $(4') \times (1.7') \times (0.668) = 0.02 \text{ TONS}$

WALL FIGS (CONT.)

> WF-4

LENGTH: $(17') + (55.5') = 37.3 \text{ CY}$

EXTRA LENGTH FOR REINF. N-MAT FIGS =

DIM: $(6') \times (3.2')$

#7 CONT.

#5 @ 12" O.C. STIRR.

#3-7 = $10(57.5) \times (2.044) + [36(6') \times (1.043)] = 0.65 \text{ TONS}$

> WF-5

LENGTH: $(20.9') + (12.4') + (143.4') = 379 \text{ CY}$

EXTRA LENGTH FOR REINF. THRU MATS: $(34.5') + (18')$

DIM: $(8') \times (3.5')$

#7 CONT.

#6 @ 12" O.C. STIRR.

#3-7 = $11(473) \times (2.044) + [47(8') \times (1.500)] = 7.2 \text{ TONS}$

> WF-6 * EXTEND BARS 10" INTO NEXT FIG

LENGTH: $(53') = 55.0 \text{ CY}$

DIM: $(8') \times (2.5')$

#7 CONT.

#5 @ 12" O.C. THRU DZ STIRRUPS

#3-7 = $18(73') \times (2.044) + [53(8') \times (1.043)] = 1.56 \text{ TONS}$

SMITH

> UNDER (ARROWWAY) (VERT)

LENGTH: $(10.7') + (10.8') + (18') = 5.9 \text{ CY}$

$(2') \times (2') \times$ LENGTH

#4 CONT. W/ LENGTH

#3-7 = $(4') \times (39.5') \times (0.668) = 0.04 \text{ TONS}$

Structural Estimate Calculations

BASEMENT WALLS (cont.) -- 4,000 PSI
 **X ASSUMING ALL FOUNDATION WALL HTS ARE 34'-0"
 DUE TO CONCRETS IN STR + RECH PILLS

> 1'-8" WALL
 HEIGHT = 24'
 LENGTH: $(8.6') + (3.6') + (12') + (6') + (24') + (28') + (8')$
 $(18') + (20') + (5') + (17') + (37') + (14') + (15') + (6.5') + (5')$
 = 422.4'

REINF: #10 @ 12" O.C. VERT I.F.
 #6 @ 12" O.C. VERT O.F.
 #5 @ 12" O.C. INTXZ BOTH FACES

DIMENS: #10 @ 12" O.C. OF 36 S' IN LENGTH
 #6 @ 12" O.C. I.F.

#3-7 = $(422 \times 34 \times 1.502) + [34(422.4 \times 1.043)] + [427(6.5 \times 1.502)] = 206 \text{ TONS}$

#6-18 = $422(34 \times 1.303) + [427(6.5 \times 1.303)] = 374 \text{ TONS}$

896 CY

BASEMENT WALLS -- 4,000 PSI
 **ASSUMING REMAIN WALL REINF IS SAME AS TOP

> 2'-0" SOUTH WALL / SE WALL AROUND LOW SUB
 LENGTH: $(18.8') \times 6$
 $(17.5') + (12') + (2.8') + (14.0') + (20.8') + (41')$
 = 202.7'

HEIGHT = 24' / 34'

REINF: #6 @ 12" O.C. VERT O.F.
 #10 @ 6" O.C. " I.F.
 #5 @ 12" O.C. INTXZ BOTH FACES

DIMENS: #10 @ 6" O.C. O.F.
 #6 @ 12" O.C. I.F.

#3-7 = $[202(34 \times 1.502)] + [251(202.7 \times 1.043)] + [202(4 \times 1.502)] = 13 \text{ TONS}$

#8-18 = $[404(34 \times 1.303)] + [404(4 \times 1.303)] = 33 \text{ TONS}$

> APPEARWAYS

- SOUTH WALL
 EASTER LENGTH: $(12') + (12') + (20')$
 HEIGHT = 24.2'

REINF: #6 @ 12" O.C. VERT I.F.
 #10 @ 6" O.C. INTXZ BOTH FACES

INNER LENGTH = 20'
 I.F.

REINF: #5 @ 12" O.C. EA WAY BOTH FACES
 (ADD EXTRA 1" VERT IN ANCHOR INTO ETO)

#3-7 = $[4(22 \times 1.502)] + [4(14.2 \times 1.502)] + [30(11 \times 1.502)] + [10(22 \times 1.043)] + [11(20 \times 1.043)] = 32 \text{ TONS}$

#8-18 = $[8(14 \times 1.303)] = 5.6 \text{ TONS}$

- EAST WALL
 LOWER LENGTH: $(4') + (25.6') + (31') + 30.6'$
 HT = 24.4'

#3-7 = 217.7 CY

INNER LENGTH = 12.5'
 #3-#7 = 188 TONS

ALL REINF SAME AS SOUTH APPEARWAY
 #8-18 = 3.23 TONS

1476 CY

Structural Estimate Calculations

SHEAR WALLS - 5,000 PSI

→ SWI. → 101 CY

- 20.7" THICK
- HEIGHT = (177'-8") - (48'-6") = 131'

- #5 @ 12" O.C. BOTH FACES HORIZ. & VERT

- 20" THK BASEMENT WALL (TO EL 85'-8")

- 16 #9 CHOKED BARS (EL 48'-6" TO EL 15'-0") = 66.5'

- 12 #9 " (EL 15'-0" TO 144'-4") = 29.3'

- 8 #9 " (144'-4" TO 149'-8") = 35.4'

#3-7 = $[131(20.7)(0.43)] + [2(131)(1.043)] = 2.85 \text{ TONS}$

#8-18 = $[16(66.5)(3.4)] + [12(29.3)(3.4)] + [8(35.4)(3.4)] = 5.1 \text{ TONS}$

→ SW2 → 48.5 CY

- 11" THK
- 10'-0" WIDE
- HEIGHT = SW1

- REG REINF = SW1

- 20" THK BASEMENT WALL (TO EL 85'-8")

- 12 #9
- 10 #9
- 8 #9

#3-7 = $[131(10)(1.043)] + [10(131)(1.043)] = 1.4 \text{ TONS}$

#8-18 = $[12(66.5)(3.4)] + [10(29.3)(3.4)] + [8(35.4)(3.4)] = 2.5 \text{ TONS}$

INT. BASEMENT WALLS + THICKENED SLAB

GENERAL

- 160T (20')

- EXTRA 5" THICKNESS

- (1.5') + (WIDTH OF WALL)

- 3 #4 CONTINUOUS

LENGTH: (10') + (22') + (21') + (31') + (20.5') + (4(19')) + (15.5') + (15.5')

(8' WIDTH) + 4(15') + (29') + (15') + (13.5') + (43.5') + (50') + (50') + (30')

+ (24') + (20') + (24') + (18') = 578.5'

LENGTH: (10') + (16') + (6') + (37') + (10') + (23.5') + (12') + (12.5') + (10.5') = 177.5'

[1.5' WIDTH CONC.]

CMU: HORIZONTAL JOINT REINF @ 16" O.C. VERTICALLY

#4 @ 24" O.C. VERT ... CELLS GRADED SOLID

CONC.: 2 #4 @ 12" O.C. BOTH WAYS

CONC THICK SLAB: $(\frac{1}{2}) [25(137.5) + 216(578.5)] = 24.6 \text{ CY}$

WALLS: $(137.5)(20)(1') = 102 \text{ CY}$

CONC REINF → 0.72 TONS

#3-7 = $[3(578.5 + 137.5)(0.668)] + [2(138(20)(0.668) + 20(137.5)(0.668))] = 37 \text{ TONS}$

CMU AREA: $(578.5)(20) = 11,570 \text{ SF}$

REINF (#4) $(20)(\frac{578.5}{2}) = 5785 \text{ LF}$

Structural Estimate Calculations

SHEAR WALLS (CONT.)

> SW7
 - 7" THK
 - HEIGHT = $(173'-8") - (44'-0") = 129.7'$
 - 12'-10" WIDE
 - #5 @ 12" OC BOTH FACES, BOTH WAYS
 - 12 #9 CHORD BARS (EL 44'-0" TO EL 15'-0")
 - 8 #9 " (EL 15'-0" TO EL 173'-8")
 CONC = 61.5 CY
 #3-7 = $1.7(1.05) = 1.8$ TONS
 #8-18 = $3.03(1.05) = 3.2$ TONS

> SW8
 - 12" THK
 - 16'-0" WIDE
 - HEIGHT = $(159'-0") - (44'-0") = 115'$
 - #5 @ 12" OC BOTH FACES, BOTH WAYS
 - 12 #9 CHORD BARS (EL 44'-0" TO EL 144'-4")
 - 8 #9 " (EL 144'-4" TO EL 159'-0")
 CONC = $115(16) = 68.1$ CY
 #3-7 = 1.95 TONS
 #8-18 = 2.19 TONS

SHEAR WALLS (CONT.)

> SW3
 - IDENTICAL TO SW2
 #3-7 = 1.4 TONS
 #8-18 = 2.5 TONS
 48.5 CY

> SW4 (x 2)
 - 12" THK
 - 20" THK BASEMENT WALL (TO EL. 95'-8")
 - HEIGHT = $(179'-8") - (44'-0") = 135.7'$
 - REINF SIM TO SW1 (CHORD BARS ALSO SIMILAR TO SW1)
 - EXTRA #4 @ 12" OC HORIZ
 #3-7 = $2.85(1.05) = 3.0$ TONS
 #8-18 = $3.1(1.05) = 3.3$ TONS
 105 CY

> SW5
 - SIM TO SW2, EXCEPT:
 Δ WIDTH = 1'-8"
 Δ T/16 EL = 44'-0"
 CONC = $48.5(1.21) = 58.7$ CY
 #3-7 = $1.4(1.21) = 1.7$ TONS
 #8-18 = $2.5(1.21) = 3.03$ TONS

> SW6
 - IDENTICAL TO SW5
 58.7 CY
 #3-7 = 1.7 TONS
 #8-18 = 3.03 TONS

Structural Estimate Calculations

SHEAR WALLS (CONT.)

> SW11

- SIMILAR TO SW10, EXCEPT:
- Δ WIDTH = 16'-0"

CONC = $16.5' (16') = 267 \text{ CY}$

#3-#7 = $1.48 (1.8) = 2.68 \text{ TONS}$

#8-#18 = $1.48 (2.13) = 3.23 \text{ TONS}$

SHEAR WALLS (CONT.)

> SW9

- 12" THK
- 10'-9" WIDE
- HEIGHT = $(69'-0") - (41'-6") = 27'-6"$
- #5 @ 12" O.C. BOTH FACES, BOTH WAYS
- 16 #9 CHRD BARS (EL 49'-6" TO EL 129'-8")
- 12 #9 " (EL 129'-8" TO EL 151'-0")

CONC = $(10.75') (27.5') = 296 \text{ CY}$

#3-#7 = 1.7 TONS

#8-#18 = 2.05 TONS

> SW10

- SIMILAR TO SW11, EXCEPT:
- Δ T/FIG EL = 42'-6"

CONC = $(10.75') (16.5') = 177 \text{ CY}$

#3-#7 = $1.7 (1.064) = 1.81 \text{ TONS}$

#8-#18 = $2.05 (1.064) = 2.18 \text{ TONS}$

Structural Estimate Calculations

* ASSUMING DOWEL LENGTH = 4

COLUMNS (CONT.)

> C-2 (x2)
 CONC: $\frac{1}{4}(3.35)^2(21.8) + \frac{1}{4}(2.0)^2(103.3) = 411 \text{ CF} = (15.2 \text{ CY}) \times 2 = 30.4 \text{ CY}$

- STL:
 Δ#3-#7:
 DOWELS: 8#7 → 65 LB
 TIES: $15(\frac{1}{4}(2.0)^2)(0.688 \text{ TIE}) = 65 \text{ LB}$
 $8(\frac{1}{4}(1.85)^2)(0.688 \text{ TIE}) = 203 \text{ LB}$
 } 1433 LB = 0.433 TONS

Δ#8-#18:
 LONG: $10(37.2)(3.4 \text{ TIE}) = 1265 \text{ LB}$
 $8(88)(3.4 \text{ TIE}) = 2394 \text{ LB}$
 } 3,659 LB = 3.659 TONS

> C-3 (x9)
 CONC: $(1.83)(2.17)(110.5) = 439 \text{ CF} = (6.3 \text{ CY}) \times 9 = 117 \text{ CY}$

- STL:
 Δ#3-#7:
 DOWELS: 8#7 → 65 LB
 TIES: $6(1+15)(2.17)(0.688 \text{ TIE}) = 652 \text{ LB}$
 $7(17 \text{ LB}) = 6453 \text{ LB} = 3.23 \text{ TONS}$

Δ#8-#18:
 LONG: $8(3400 \text{ TIE})(66.5) = 1809 \text{ LB}$
 $8(2.67 \text{ TIE})(44) = 940 \text{ LB}$
 $7(719 \text{ LB}) = 2471 \text{ LB} = 12.4 \text{ TONS}$

> C-4 (x1)
 CONC: $(1.83)(2.17)(20.85) = 82.7 \text{ CF} = 349 \text{ CF} = 12.9 \text{ CY}$

- STL:
 Δ#3-#7:
 DOWELS: $12(4)(2.044) = 98 \text{ LB}$
 TIES: $14(4(83) + (217))(0.688 \text{ TIE}) = 154 \text{ LB}$
 $65(3(1.5) + 3(2))(0.688 \text{ TIE}) = 470 \text{ LB}$
 } 722 LB = 0.361 TONS

Δ#8-#18:
 LONG: $12(3.4 \text{ TIE})(20.83) = 850 \text{ LB}$
 $8(3.4)(74) = 2013 \text{ LB}$
 $8(2.67)(14.67) = 313$
 } 3176 LB = 1.588 TONS

COLUMNS - 5,000 PSI

C-1: 10'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-2: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-3: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-4: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-5: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-6: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-7: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-8: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-9: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-10: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-11: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-12: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-13: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-14: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

C-15: 14'-6" x 14'-6" x 18'-6" (1#11, 1#11, 1#11)

> C-1
 CONC: $(1.83)(1.83)(116.5) = 370 \text{ CF} = 14.4 \text{ CY}$

- STL:
 Δ#3-#7:
 DOWELS: 8#7 → $8(4)(2.044 \text{ TIE}) = 65 \text{ LB}$
 TIES: $6(1.83)(6)(0.688 \text{ TIE}) = 499 \text{ LB}$
 $15(1.83)(6)(0.688 \text{ TIE}) = 110 \text{ LB}$
 } 674 LB = 0.337 TONS

Δ#8-#18:
 LONG: $8(72.5)(3.40 \text{ TIE}) = 1972 \text{ LB}$
 $8(44)(2.67 \text{ TIE}) = 940 \text{ LB}$
 } 2912 LB = 1.456 TONS

Structural Estimate Calculations

COLUMNS (CONT.)

> C-7 (x 2)
 - CONC: $(1.83) \cdot (2.17) \cdot (115.7) = 460 \text{ CF} \cdot 2 = 920 \text{ CF} = 34.1 \text{ CY}$

- STL:
 Δ #3-#7:
 Dowels: 98 LB
 TIES: $101 \cdot (4.683) \cdot (4 \cdot (2.17)) + 15 \cdot (4.145) \cdot (2 \cdot (2.17)) = 1796'$
 $1796' \cdot (0.688 \text{ TLF}) = 1236 \text{ LB} \cdot 2 = 2472 + 248 = 2668 \text{ LB} = 1.33 \text{ TONS}$

Δ #8-#18:
 LONG: $12 \cdot (4.303 \text{ PUF}) \cdot (57) = 2943$
 $12 \cdot (3.4) \cdot (44) = 1795$
 $8 \cdot (3.4) \cdot (14.7) = 400$
 $2943 + 1795 + 400 = 5138 \text{ LB} \cdot 2 = 10,276 \text{ LB} = 5.1 \text{ TONS}$

> C-8 (x 5)
 - CONC: $(2.17) \cdot (2.17) \cdot (116.2) = 547 \text{ CF} \cdot 5 = 2736 \text{ CF} = 101 \text{ CY}$

- STL:
 Δ #3-#7:
 Dowels: 65 LB
 TIES: $85 \cdot (6 \cdot (2.17)) \cdot (0.688 \text{ TLF}) = 713 \text{ LB}$
 $(806 \text{ LB}) \cdot 5 = 4040 \text{ LB} = 2.02 \text{ TONS}$

Δ #8-#18:
 LONG: $8 \cdot (4.303 \text{ PUF}) \cdot (57.5) = 1979$
 $12 \cdot (3.4) \cdot (44) = 1597$
 $1979 + 1597 = 3576 \text{ LB} \cdot 5 = 17,880 \text{ LB} = 9.0 \text{ TONS}$

COLUMNS (CONT.)

> C-5 (x 11)
 - CONC: $(2.17) \cdot (2.17) \cdot (111) = 523 \text{ CF} \cdot 11 = 213 \text{ CY}$

- STL:
 Δ #3-#7:
 Dowels: 98 LB
 TIES: $35 \cdot (8 \cdot (2.17)) \cdot (0.688 \text{ TLF}) = 418 \text{ LB}$
 $30 \cdot (6 \cdot (2.17)) \cdot (0.688) = 269 \text{ LB}$
 $15 \cdot (3.4) \cdot (11) = 134 \text{ LB}$
 $418 + 269 + 134 = 821 \text{ LB} \cdot 11 = 10,109 \text{ LB} = 5.05 \text{ TONS}$

Δ #8-#18:
 LONG: $12 \cdot (3.4 \text{ PUF}) \cdot (52.3) = 2124 \text{ LB}$
 $8 \cdot (3.4 \text{ PUF}) \cdot (58.67) = 1576 \text{ LB}$
 $2124 + 1576 = 3700 \text{ LB} \cdot 11 = 41,030 \text{ LB} = 20.5 \text{ TONS}$

> C-6 (x 9)
 - CONC: $(2.17) \cdot (2.17) \cdot (116.5) = 549 \text{ CF} \cdot 9 = 183 \text{ CY}$

- STL:
 Δ #3-#7:
 Dowels: 98 LB
 TIES: $4 \cdot (8 \cdot (2.17)) \cdot (0.688 \text{ TLF}) + 821 \text{ LB} = 869 \text{ LB}$
 $(967 \text{ LB}) \cdot 9 = 8703 \text{ LB} = 4.35 \text{ TONS}$

Δ #8-#18:
 LONG: $12 \cdot (4.303 \text{ PUF}) \cdot (57.83) = 2986 \text{ LB}$
 $12 \cdot (3.4 \text{ PUF}) \cdot (44) = 1795 \text{ LB}$
 $8 \cdot (3.4) \cdot (14.5) = 395 \text{ LB}$
 $2986 + 1795 + 395 = 5176 \text{ LB} \cdot 9 = 46,584 \text{ LB} = 23.3 \text{ TONS}$

Structural Estimate Calculations

COLS (CONT.)

> C-11 (x 2)
 CONC: $(1.83)(2.17)(27.4) = 506 \text{ CF}$ } 1012 CF = 37.5 CY

- STL:
 #3 #7
 Dowels: 65 LB
 TIES: $90(32.17) + 2(183) = 2913 \text{ LF}$ } $(0.688 \text{ PLF}) = 743$ } 808 LB = 0.81 TONS

#8 #18
 LONG: $8(3.4 \text{ PLF}) = 27.2$ } 1869 } $(31.23) = 6246 \text{ LB} = 3.1 \text{ TONS}$
 $8(2.67) = 21.36$ } 1254

> C-12
 CONC: $(1.72)(14.6) = 24.2 \text{ CF} = 1.1 \text{ CY}$

- STL:
 #3 #7
 Dowels: $8(4) = 32 \text{ PLF}$ } 48 LB
 TIES: $15(3.5) = 52.5 \text{ LF}$ } 93 LB } 141 LB = 0.07 TONS

#8 #18
 LONG: $8(2.67) = 21.36$ } 312 LB = 0.16 TONS

COLUMNS (CONT.)

> C-9 (x 6)
 CONC: $(2.17)(116.5) = 549 \text{ CF}$ } 6 = 3292 CF = 122 CY

- STL:
 #3 #7:
 Dowels: $4(16) = 64 \text{ PLF}$ } 140 LB
 TIES: $30(10)(2.17) = 651 \text{ LF}$ } $(0.688 \text{ PLF}) = 448$ } 1172 LB = 8832 LB = 4.4 TONS
 $74(8)(2.17) = 1283 \text{ LF}$ } 884

#8 #18
 LONG: $16(5.33 \text{ PLF}) = 85.28$ } 3672 } $(44.83 \text{ LB}) = 46,032 \text{ LB} = 23 \text{ TONS}$
 $12(5.33) = 64$ } 2805
 $12(2.4) = 28.8$ } 1195

> C-10 (x 9)
 CONC: $(4.60 \text{ CF}) = 4186 \text{ CF} = 155 \text{ CY}$

- STL:
 #3 #7
 Dowels: $10(4) = 40 \text{ PLF}$ } 82 LB
 TIES: $83(2)(183) = 30138 \text{ LF}$ } $(0.688 \text{ PLF}) = 655 \text{ LB}$ } 667 LB = 6,903 LB = 3.15 TONS

#8 #18
 LONG: $10(4.503 \text{ PLF}) = 45.03$ } 2487 } $(44.83 \text{ LB}) = 40,347 \text{ LB} = 20.2 \text{ TONS}$
 $10(3.4) = 34$ } 1996

Structural Estimate Calculations

ELEVATED SLABS

1st - 2nd FLOORS - 3rd FLOOR ADJUSTED FOR ATTICUM

AREA: $28,550 \text{ SF} - (57 \times 14) - (20 \text{ SF}) - (7 \times 17) - (20 \text{ SF}) - (5 \times 4.4) - (6 \times 57) - (10.4 \times 8.6) - (3.8 \times 5.4) - (9 \times 9.8) - (9 \times 4) - (50 \text{ SF})$
 $= 28,100 \text{ SF} \Rightarrow 998 \text{ CF} \rightarrow \text{ADJUSTED} = 968 \text{ CF}$

THICKNESS = 11.5"

OVERALL DIM: (115') x (242.7')

BOTTOM MAT: #5 @ 10" O.C. EA WAY $\Rightarrow 18,19 \text{ TONS} + 18,14 \text{ TONS} = 36.3 \text{ TONS}$

ADDITIONAL REINF: $\rightarrow \text{ADJUSTED} = 35.2 \text{ TONS}$

#6 @ 4'
 $22, 20, 13, 8, 10, 13, 16, 10, 12, 12, 17, 36, 22, 25, 10, 12, 11, 25, 8 = 440$
 $440(4)(1502) = 32 \text{ TONS}$

#6 @ 7'
 $3(10), 10(14), 12(25), 8, 4, 8, 8 = 828$
 $828(7)(1502) = 4.35 \text{ TONS}$

#6 @ 10'
 $10(18)(1502) = 0.14 \text{ TONS}$

#6 @ 7.5'
 $8(155), 10(14), 4 = 524$
 $524(7.5)(1502) = 5.11 \text{ TONS}$

#8 @ 6'
 $10(6), 4(14), 8, 6, 4, 6 = 236$
 $236(6)(1502) = 1.89 \text{ TONS}$

#8 @ 8'
 $10(22) = 220$
 $320(8)(1502) = 3.47 \text{ TONS}$

#8 @ 10'
 $6(18)(1502) = 0.14 \text{ TONS}$

#8 @ 10'
 $18, 20, 12, 15, 11 = 80$
 $80(10)(1502) = 0.11 \text{ TONS}$

ELEVATED SLABS (CONT.)

AREA OF ATTICUM SPACE:
 $(22.5 \times 10) + (25.5 \times 10) - (36 \times 10) = 840 \text{ SF}$

$\frac{840 \text{ SF}}{28,100 \text{ SF}} = 0.0299 = 3\% \text{ OF TOTAL SLAB AREA}$

COLS (CONT.)

> C-13
 - CONC: $(1.83)^2(116.5) = 390 \text{ CF} = 14.5 \text{ CY}$

- STL
 Δ #3 #7
 DIAPHS = 98 LB
 TIES: $83(8)(183)(0.688) = 836 \text{ LB}$ } $934 \text{ LB} = 0.5 \text{ TONS}$
 Δ #8 #18
 LONG: $12(5.313)(57.2) = 3685 \text{ LB}$
 $12(4.303)(58.7) = 3031 \text{ LB}$ } $6716 \text{ LB} = 3.4 \text{ TONS}$

> C-14
 - CONC: $(1)(2)(115) = 230 \text{ CF} = 8.5 \text{ CY}$

- STL
 Δ #3 #7
 DIAPHS = 65 LB
 TIES: $115(9)(183)(0.688) = 712 \text{ LB}$ } $777 \text{ LB} = 0.4 \text{ TONS}$
 Δ #8 #18
 LONG: $8(2.67)(115) = 2456 \text{ LB} = 1.2 \text{ TONS}$

> C-15 (x3)
 - CONC: $(1.83)(2.17)(91.2) = 164 \text{ CF} = 18.2 \text{ CY}$

- STL
 Δ #3 #7:
 DIAPHS: $10(4)(104) = 81.8 \text{ LB}$
 TIES: $28(20.7)(31.83)(0.688) = 231 \text{ LB}$ } $313 \text{ LB} = 0.147 \text{ TONS}$
 Δ #8 #18
 LONG: $10(4.303)(41.2) = 1773 \text{ LB} = 2.66 \text{ TONS}$